## Hesketh



Civil & Traffic Engineers • Surveyors • Planners • Landscape Architects

F. A. Hesketh & Associates, Inc.

August 14, 2015

**Draft 3** 

Mr. Jack Yang Artesan Development, LLC

Via: E-mail jyang628@gmail.com

**RE:** Meadowbrook Gardens

**Meadowbrook Lane** 

Mansfield, CT

**Traffic Impact Report** 

Our File: 15131

Dear Mr. Yang:

This report documents the findings of a traffic impact study for a proposed expansion of the existing Meadowbrook Gardens apartment complex located on Meadowbrook Lane in the Town of Mansfield, Connecticut. Meadowbrook Gardens is located on the south side of Meadowbrook Lane, west of Pollack Road and east of Circle Drive. The site location is presented in Figure 1.

The purpose of this study is to estimate the amount of traffic that will be generated by the proposed expansion and to determine its impact on the adjacent roadway network. It is anticipated that this report will accompany an application to the Town of Mansfield for site plan approval.

## **Proposed Development**

The existing Meadowbrook Gardens development is approved for a total of 50 apartment units. A total of 102 parking spaces are provided on site. The site has access to Meadowbrook Lane by way of a single driveway. The driveway provides a single entering and exiting lane, separated by a raised landscaped median. The driveway approach operates under stop sign control.

The current proposal is for an expansion of up to 40 new apartment units on property adjacent to, and west of the existing site. Access to the new units will by way of a new driveway to Meadowbrook Lane. An internal access will be provided to the existing development as well. The proposed driveway will provide a single entering and exiting lane separated by a raised landscaped median. The driveway approach will operate under stop sign control. Upon approval and completion of construction the combined

developments will consist of a total of up to 90 apartment units with a total floor are of 82,000 s.f. A total of 185 parking spaces will be provided.

## **Description of Area**

Meadowbrook Lane is a town maintained roadway that originates at an un-signalized intersection with Mansfield City Road approximately 1,000 feet west of the subject site. Meadowbrook Lane extends in an easterly direction to an all stop sign controlled intersection with Circle Drive, then past the subject site to an intersection with Pollack Road and Adeline Place. Meadowbrook Lane continues easterly to its terminus at Conantville Road. The roadway continues as Conantville Road to its terminus at Route 195, Storrs Road. Meadowbrook Lane provides approximately 18 to 22 feet of pavement with a single travel lane in each direction of travel, separated by a painted double yellow centerline. Speed bumps are located along the length of the roadway. The roadway is posted at 25 miles per hour. Land use along the roadway is residential.

Pollack Road originates at an un-signalized intersection with Meadowbrook lane and Adeline Place and extends in a southwesterly direction a distance of approximately 750 feet to its terminus at Conantville Road. The intersection of Pollack Road with Meadowbrook Lane operates with stop sign control on the side street approaches.

Conantville Road originates at an un-signalized intersection with S.R. 632 (North Frontage Road) and the westbound off ramp from Route 6. Conantville Road extends in a northerly direction past the intersection with Pollack Road to its intersection with Meadowbrook Road. Conantville Road continues in a northeasterly direction to its terminus at Route 195.

## **Background Traffic Data**

The Connecticut Department of Transportation maintains a continuous count program of traffic volumes on the Connecticut state highway system and some local roadways. Included in the state's database are counts on Conantville Road, west of Route 195, conducted during October 2014, and one on Conantville Road, north of Pollack Road conducted during October 2011. The count west of Route 195 indicates that Conantville Road carries an average daily traffic volume (ADT) of 800 vehicles with a morning peak hour volume of 73 vehicles (8:00 a.m.) and an afternoon peak hour volume of 83 vehicles (4:00 p.m.). The count north of Pollock Road indicates an ADT of 550 vehicles, with a morning peak hour volume of 52 vehicles (8:00 a.m.) and an afternoon peak hour volume of 63 vehicles (3:00 p.m.). The ConnDOT count data are presented in Tables 1 and 2.

In order to verify and update the ConnDOT counts, our office arranged for the installation of an automated traffic volume counter on Meadowbrook Lane, immediately west of the existing Meadowbrook Gardens Driveway, from June 25, 2015 through July

1, 2015. The count indicates a weekday average volume of 450 vehicles, with a morning peak hour volume of 29 vehicles (9:00 a.m.) and a p.m. peak hour volume of 45 vehicles (4:00 p.m.). The count is presented in Table 3.

In addition to the automated counts described above, movement turning movement counts were conducted during the morning and afternoon commuter peak periods at the intersections of Meadowbrook Road with Pollack Road / Adeline Place and at Route 195 with Conantville Road. Copies of these counts are provided in the appendix. The observed traffic volumes for the a.m. and p.m. peak hours are presented in Figure 2.

A review of the files of the Town of Mansfield and the Office of the State Traffic Administration (OSTA) did not reveal any traffic impact reports that need to be considered as part of the background traffic for this proposal.

A 2% per year growth rate to a design year of 2017, or a total increase of 4%, was applied to the volumes in Figure 2. The resultant volumes are the 2017 background traffic volume. These volumes are presented in Figure 3.

## **Site Generated Traffic and Traffic Assignment**

The proposed development is to consist of a total of up to 40 new apartment units. The combined development will consist of a total of up to 90 apartment units. The trip generation for the development was calculated utilizing the ITE Trip Generation report. Included in the ITE Trip generation is land use 220 - Apartments. Applying the ITE equations to the existing 50 units and the 90 unit developments yields the following trip generations. The existing 50 units have a trip generation potential of 427 trips on a daily basis, with an a.m. peak hour of 28 trip made up of 6 entering trips and 22 exiting trips and a p.m. peak hour of 45 trips made up of 29 entering trip and 16 exiting trips. The combined 90 unit development has a trip generation potential of 669 trips on a daily basis, with an a.m. peak hour of 48 trips made up of 10 entering trips and 38 exiting trips and a p.m. peak hour of 67 trips made up of 44 entering trips and 23 exiting trips. By subtracting the trip generation of the 90 units from the existing 50 units, the trip generation for the proposed 40 units can be determined. Based on this methodology the proposed expansion can be expected to generate a total of 242 trips on a daily basis, with an a.m. peak hour of 20 trips made up of 4 entering trips and 16 exiting trips and a p.m. peak hour of 22 trips made up of 15 entering trip and 7 exiting trips. A summary of the trip generation results are presented as Table 4.

The existing development is currently under construction and not yet fully occupied. To be conservative in our analysis we have assumed that the traffic from all 90 units is new traffic. In addition, we have assumed a directional distribution consistent with the original Traffic Engineering Solutions report. That distribution has 67% of the site traffic oriented to and from the west and 67% to and from the east on Meadowbrook Lane.

This distribution overestimates the site generated traffic by 34% but accounts for any variation in distribution.

Figure 4 presents the trip distribution used in this report. Figure 5 presents the site generated traffic based on the combined development traffic from Table 4 and the distribution in Figure 4. By adding the site generated traffic in Figure 5 to the background traffic volumes in Figure 3, the combined traffic volumes upon completion of the development can be determined. These volumes are presented in Figure 6.

## **Capacity Analysis and Traffic Impact**

Capacity analyses were completed for the background and combined traffic volumes at the following intersections:

- Route 195 at Conantville Road
- Meadowbrook Lane at Pollack Road and Adeline Place
- Meadowbrook Lane and the Meadowbrook Garden Driveway

The analyses were completed to determine the operational condition of the intersections before and after the introduction of site traffic, thereby determining the impact of site traffic on the intersection. The methodology employed is found in the Highway Capacity Manual (HCM), published by the Transportation Research Board. This methodology results in an intersection rating in terms of "Level of Service" (LOS), which defines the amount of delay expected at the intersection. The Levels of service results are presented in Table 5. A brief description of each intersection is presented here.

Route 195 at Conantville Road – This is an existing un-signalized "T" intersection with Route 195 (Storrs Road) oriented in the north/south direction. Conantville Road approaches from the west. The northbound and southbound Route 195 approaches each provide a single lane approach and operate free of control. The Conantville Road approach provides a single lane and operates under stop sign control. An analysis indicates that the northbound and southbound Route 195 approaches operate at a LOS A during peak hours under the background traffic volume conditions. The Conantville Road approach operates at a LOS B during the morning peak hour and at a LOS C during the p.m. peak hour under the background conditions. With the introduction of the site generated traffic the Route 195 approaches will continue to operate at a LOS A. The Conantville Road approach will operte at a LOS C during the a.m. peak hour and at a LOS D during the p.m. peak hour. The increase in average vehicular delay is approximately 1 second during the morning peak hour and approximately 3 seconds during the p.m. peak hour.

**Meadowbrook Lane at Pollack Road / Adeline Place -** This is an existing four way un-signalized intersection. Meadowbrook Lane is oriented in the east/west direction. Meadowbrook lane provides a single lane on each approach and operates free of

control. Pollack Road approaches from the south, provides a single lane approach and operates under stop sign control. Adeline lane approaches from the north, provides a single lane approach and operates under stop sign control. An analysis indicates that all approaches will operate at a LOS A during peak hours under the background and combined traffic volume conditions.

Meadowbrook Lane at Meadowbrook Gardens Driveway – There is one existing and one proposed site driveway. For purposes of this analysis we have assumed a single driveway. Meadowbrook Lane lies in an east/west orientation. Meadowbrook lane provides a single lane on each approach and operates free of control. The Meadowbrook Gardens Driveway approaches from the south and provides a single lane approach and operates under stop sign control. An analysis of the intersection indicates that all movements will operate at a LOS A during peak hours under the combined traffic volume conditions.

## **Site Access**

The existing development has access to Meadowbrook Lane by a single un-signalized driveway. The driveway provides 24 feet of pavement with a single 12 foot lane for both entering and exiting traffic separated by a raised landscaped median. The driveway operates under stop sign control.

The proposed site driveway will also be to Meadowbrook Lane, located approximately 350 feet west of the existing site driveway. The proposed driveway will provide 24 feet of pavement with a single 12 foot lane for both entering and exiting traffic separated by a raised landscaped median. The driveway operates under stop sign control.

The available sight distances at the two site driveways are in excess of 500 feet in each direction. The 500 foot sight distance meets the current ConnDOT criteria for an approach speed of 45 miles per hour. Meadowbrook Road is posted at 25 miles per hour. An 85% speed of 35 mph was recorded by the Town of Mansfield during August 2007.

The driveway has been designed to accommodate an SU-30 design vehicle and will be capable of providing access to emergency vehicles.

## Conclusion

The current proposal is for an expansion of up to 40 new apartment units to the existing 50 apartment unit complex known as Meadowbrook Gardens. The proposed development is projected to generate an additional 20 trips during the morning peak hour and an additional 22 trips during the p.m. peak hour. Based on this analysis, it is our professional opinion that the existing roadway network has sufficient excess

capacity and will be capable of accommodating the increase in traffic volumes associated with this proposed expansion with little or no change in the operating condition of the roadway network. The site driveway is properly designed to accommodate the anticipated driveway volumes and it will operate at acceptable levels of service. The available sight distances from the proposed site driveway meet current ConnDOT requirements for the 85% speed.

We appreciate the opportunity to provide this analysis to you. A representative from our firm will be available to present testimony in support of your application at a hearing upon your request. Please notify me of the proposed hearing schedule as soon as is practicable. If you require any additional information, please do not hesitate to contact us.

Very truly yours, F. A. HESKETH & ASSOCIATES, INC.

Scott F. Hesketh, P.E. Manager of Transportation Engineering

cc:

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TABLE 1
ConnDOT TRAFFIC VOLUMES
CONANTVILLE ROAD WEST OF ROUTE 195
STATION NO. 077 2068

		W <u>EB</u>	15-Oct-14 /EDNESD/ <u>WB</u>		<u>EB</u>	16-Oct-14 THURSDA <u>WB</u>		17-Oct-15 FRIDAY <u>EB WB Total</u>			
			110	<u>I Otal</u>	<u> </u>	110	<u>10tai</u>		<u> </u>	Total	
	12:00				1	3	4	1	4	5	
	1:00				0	1	1	0	2	2	
	2:00				0	2	2	1	2	3	
	3:00				1	1	2	0	1	1	
	4:00				1	1	2	2	1	3	
	5:00				5	Ö	5	3	i 1	4	
***************************************	6:00	····			10	6	16	14	9	23	
	7:00				24	31	55	31	33	64	
	8:00	43	30		32	40	72	٠.	•	01	
***************************************	9:00	27	32	59	25	31	56				
	10:00	27	18	45	28	14	42				
	11:00	31	33	64	36	30	66				
	12:00	34	34	68	35	30	65				
,	1:00	26	24	50	33	23	56				
	2:00	34	36	70	31	32	63				
***************************************	3:00	47	38	85	37	43	80	<del></del>	<del></del>	······	
	4:00	40	43	83	39	39	78				
	5:00	33	20	53	34	23	57				
**********	6:00	26	33	59	27	27	54				
	7:00	17	13	30	16	18	34				
	8:00	8	14	22	12	13	25				
	9:00	6	6	12	10	8	18				
1	0:00	9	5	14	6	5	11				
	1:00	1	2	3	1	2	3				
,	1.00	•	4	3	ı	~	J				
		409	381	790	444	423	867	52	53	105	

2014 ADT = 800 for station2068 in Mansfield

TABLE 2
ConnDOT TRAFFIC VOLUMES
CONANTVILLE ROAD NORTH OF POLLACK ROAD
STATION NO. 077 2068

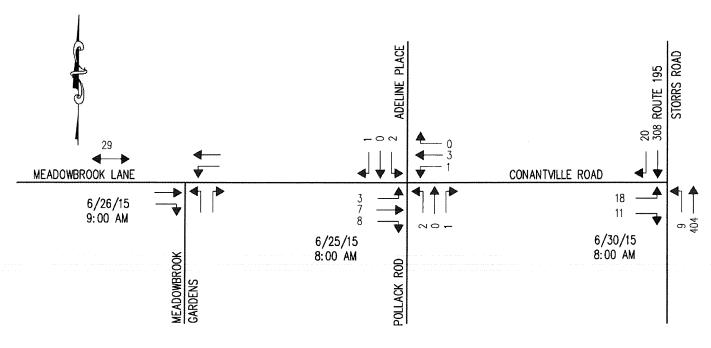
	3-Oct-11 MONDAY <u>EB</u>	4-Oct-11 TUESDAY <u>EB</u>
12:00		3
1:00		2
2:00		0
3:00		1
4:00		2
5:00		2 2
6:00		15
7:00	31	
8:00	52	
9:00	39	
10:00	30	
11:00	34	
12:00	43	
1:00	44	
2:00	36	
3:00	63	
4:00	52	
5:00	59	
6:00	27	
7:00	29	
8:00	17	
9:00	7	
10:00	6	
11:00	5	
	574	25

2011 ADT = 550 for station 87 in Mansfield

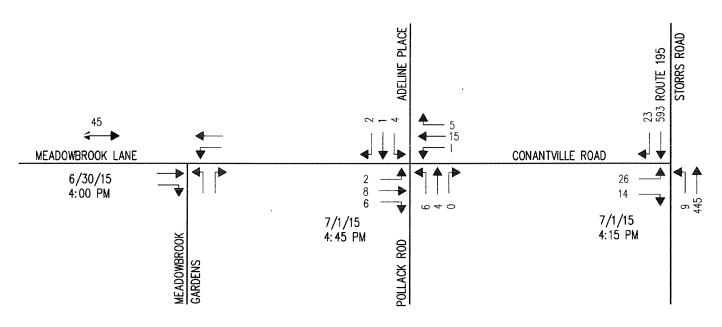
TABLE 3
F.A. HESKETH TRAFFIC VOLUME COUNT
MEADOWBROOK ROAD WEST OF SITE DRIVEWAY

	29-Jun <u>Monday</u>	30-Jun <u>Tuesday</u>	1-Jul <u>Wednesday</u>	25-Jun <u>Thursday</u>	26-Jun <u>Friday</u>	<u>Average</u>	27-Jun <u>Saturday</u>	28-Jun Sunday
12:00	0	0	1		2	1	2	2
1:00	3	3	1		0	2	4	1
2:00	0	0	0		0	0	1	2
3:00	1	2	<b>1</b>			· 1	3	1 1
4:00	1	0	0		0	0	1	2
5:00	3	1	4		3	3	3	4
6:00	8	13	10	***************************************	7	10	1	1
7:00	13	22	19		17	18	7	2
8:00	22	23	20		22	22	12	9
9:00	15	21	20		29	21	24	12
10:00	39	39	17	27	28	30	28	19
11:00	19	27	30	35	35	29	33	19
12:00	39	32	43	40	36	38	21	33
1:00	33	39	38	24	33	33	29	20
2:00	23	28	34	41	27	31	36	24
3:00	31	41	36	36	31	35	17	21
4:00	36	45	41	36	40	40	21	27
5:00	28	33	36	40	35	34	26	19
6:00	20	31		34	37	31	15	15
7:00	22	29		28	36	29	11	18
8:00	24	22		13	20	20	16	11
9:00	18	13		8	12	13	12	4
10:00	9	7		9	10	9	5	3
11:00	1	5		2	4	3	2	3
	408	476	351	373	465	451	330	272

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A.M. PEAK HOUR



P.M. PEAK HOUR



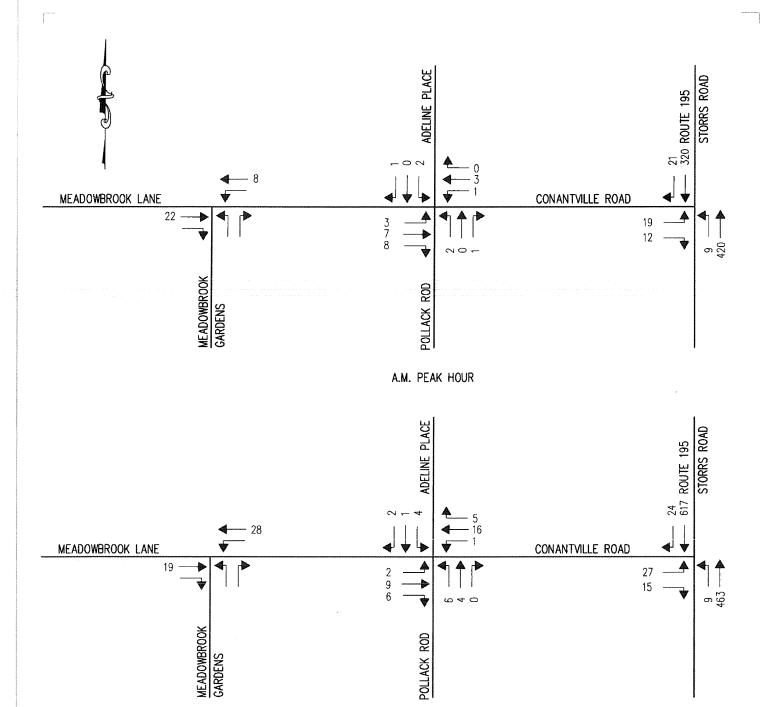


FIGURE 2 INCREASED 4% TO DESIGN YEAR 2017

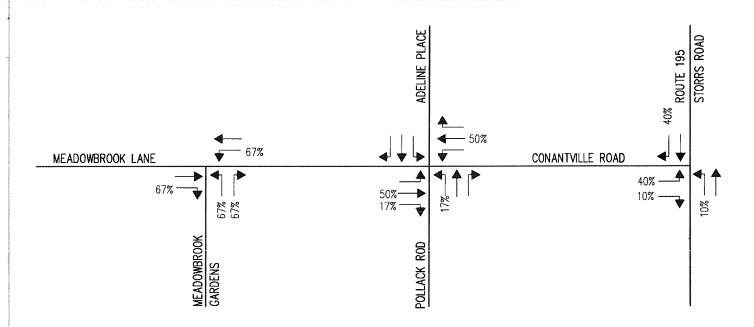


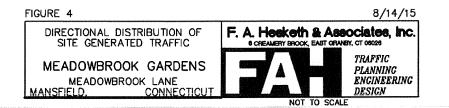
P.M. PEAK HOUR

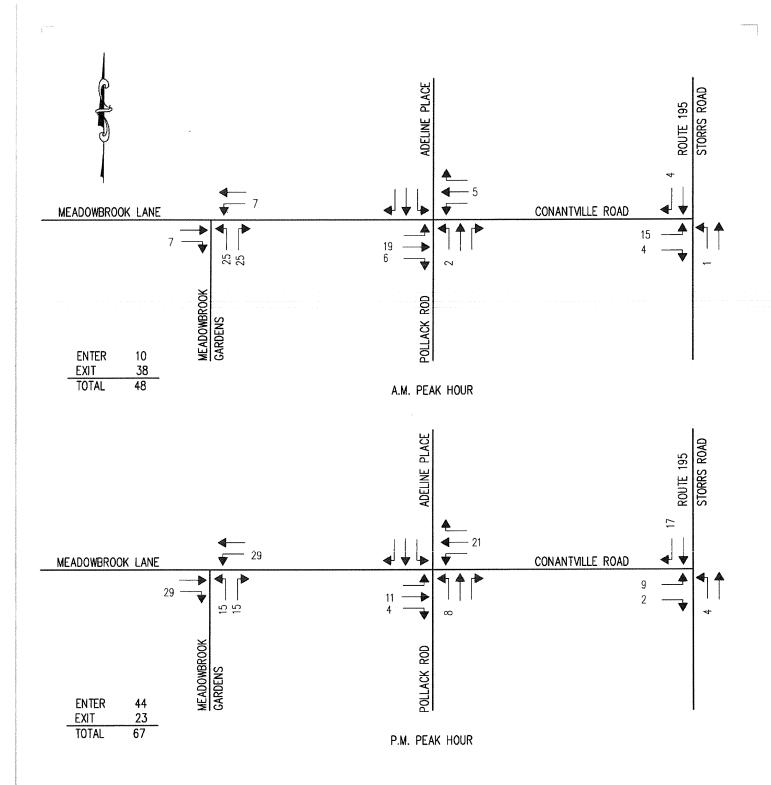
Table 4
Trip Generation
Meadowbrook Gardens
Meadowbrook Lane - Mansfield, CT

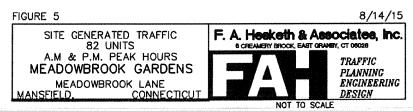
			A.	M. Peak Ho	ur	P.1	VI. Peak Ho	our
Land Use	Size	ADT	Enter	Exit	Total	Enter	Exit	Total
Combined Development Apartment	90 Units	669	10	38	48	44	23	67
Existing Development Apartment	50 Units	427	6	22	28	29	16	45
Proposed Development Apartment	40 units	242	4	16	20	15	7	22



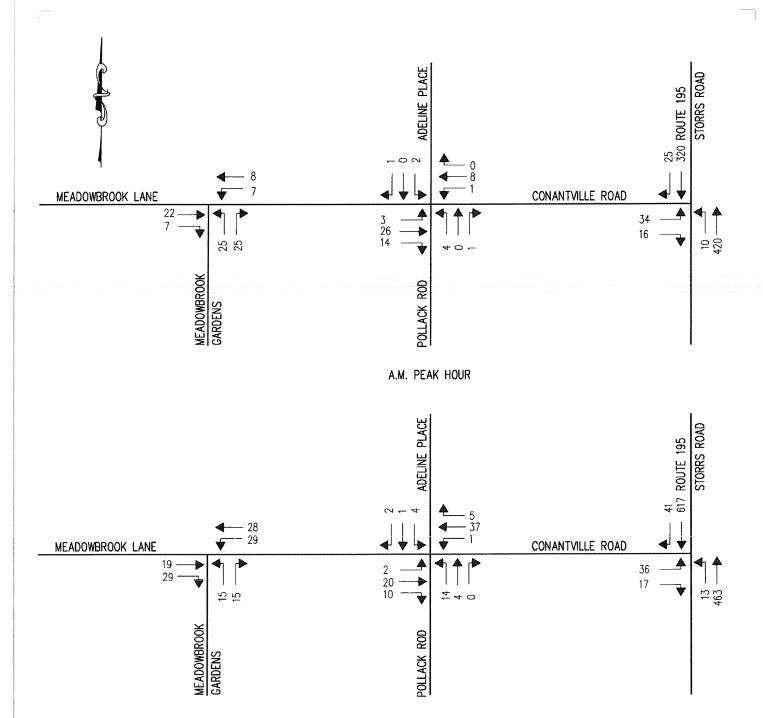




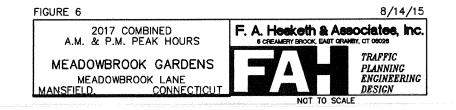




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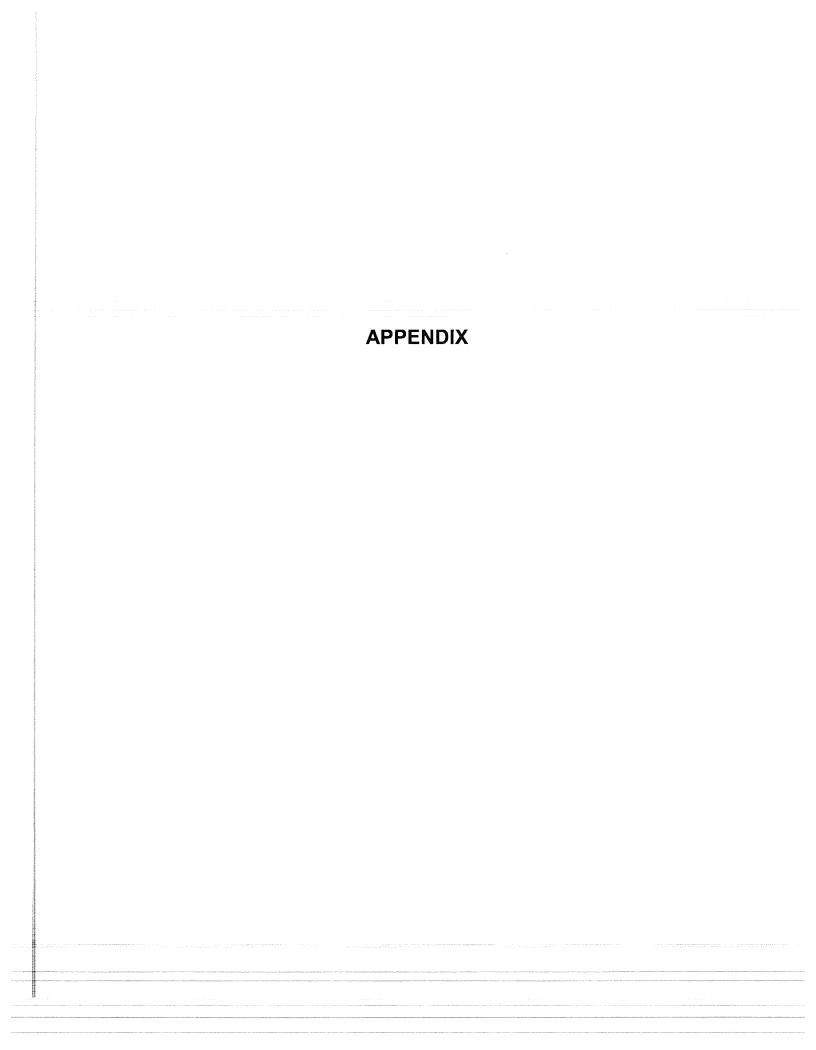
P.M. PEAK HOUR

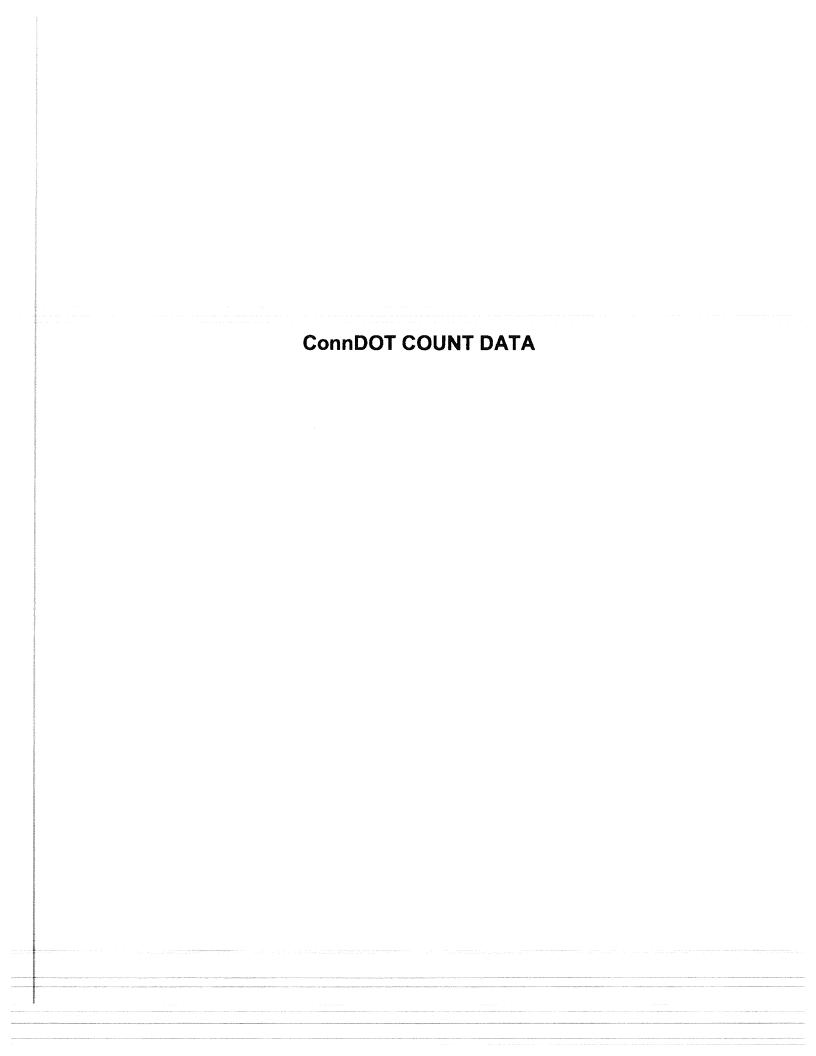


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Table 5 Level of Service Summary Meadowbrook Gardens - Mansfield, CT

	ĺ			A. M. PE	AK HOU							P. M. PE	K HOU			1
	3	Backgrou				Combine		1		Backgrou		1		Combine		1
	LOS	delay	<u>v/c</u>	Queue	LOS	delay	<u>v/c</u>	Queue	LOS	delay	<u>v/c</u>	Queue	LOS	delay	<u>v/c</u>	Queue
Route 195 at Cona	i ntville F i	Road														
NB	Α	0.3	0.01	1	Α	0.3	0.01	1	Α	0.3	0.01	1	Α	0.5	0.02	1
SB	Α	0.0	0.22	0	Α	0.0	0.22	0	Α	0.0	0.41	0	Α	0,0	0.42	0
EB	В	14.4	80.0	7	Ċ	15.5	0.14	12	С	23.2	0.19	17	D	25.8	0.25	24
Meadowbrook Land	i e at Pol	lack Roa	ad / Ade	eline Pla	ice											
NB	Α	8.6	0.00	0	Α	8.8	0.01	0	A	9.0	0.01	1	Α	9.2	0.02	2
SB	A	8.6	0.00	0	A	8.7	0.00	0	A	8.8	0.01	1	A	8.9	0.01	1
EB	Α	1.2	0.00	0	A	0.5	0.00	0	A	0.9	0.00	0	A	0.5	0.00	0
WB	А	1.8	0.00	0	Α	8.0	0.00	0	Α	0.3	0.00	0	Α	0,2	0.00	0
Meadowbrook Land	e at Mea	adowbro	ok Gar	dens Dr	iveway	•										1
NB					Α	8.8	0.05	4					Α	9.0	0.03	3
EB					Α	0.0	0.02	0					Α	0.0	0.03	0
WB					Α	3.4	0.00	0					Α	3,8	0.02	2





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# STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION BUREAU OF POLICY AND PLANNING PLANNING INVENTORY AND DATA

## TRAFFIC RECORDER DATA

TOWN OF MANSFIELD			ROUTE				DIRECTION E
CONANTVILLE ROAD #1 - I	NE OF OVERLOOK DR SUN	MON	TUE	WED	THU	FRI	SAT
DATE	0	0	0	10/15/2014	10/16/2014	10/17/2014	0
TYPE							
HOUR							
	2014 ADT		ACF = NA				
12 <b>A</b>	0	0	0	0	1	1	0
01A	0	0	0	0	0	0	0
02A	0	0	0	0	0	1	0
03A	0	0	0	0	1	0	0
04A	0	0	0	0	1	2	0
05A	0	0	0	0	5	3	0
06A	0	0	0	0	10	14	0
07A	0	0	0	0	24	31	0
08A	0	0	0	43	32	0	0
09A	0	0	0	27	25	0	0
10A	0	0	0	27	28	0	0
11A	0	0	0	31	36	0	0
12P	0	0	0	34	35	0	0
01P	0	0	0	26	33	0	0
02P	0	0	0	34	31	0	0
03P	0	0	0	47	37	0	0
04P	0	0	0	40	39	0	0
05P	0	0	0	33	34	0	0
06P	0	0	0	26	27	0	0
07P	0	0	0	17	16	0	0
08P	0	0	0	8	12	0	0
09P	0	0	0	6	10	0	0
10P	0	0	0	9	6	0	0
11P	0	0	0	1	1	0	0
	_	_	_				

409

444

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# STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION BUREAU OF POLICY AND PLANNING PLANNING INVENTORY AND DATA

## TRAFFIC RECORDER DATA

TOWN OF MANSFIELD			ROUTE				DIRECTION W
CONANTVILLE ROAD #1 DAY	- NE OF OVERLOOK DR SUN	MON	TUE	WED	THU	FRI	SAT
DATE	0	0	0	10/15/2014	10/16/2014	10/17/2014	0
TYPE							
HOUR							
	2014 ADT	= 400	ACF = NA	<b>A</b>			
	******	**					
12A	0	0	0	0	3	4	0
01A	0	0	0	0	1	2	0
02A	0	0	0	0	2	2	0
03A	0	0	0	0	1	1	0
04A	0	0	0	0	1	1	0
05A	0	0	0	0	0	1	0
06A	0	0	0	0	6	9	0
07A	0	0	0	0	31	33	0
08A	0	0	0	30	40	0	0
09A	0	0	0	32	31	0	0
10A	0	0	0	18	14	0	0
11A	0	0	0	33	30	0	0
12P	0	0	0	34	30	0	0
01P	0	0	0	24	23	0	0
02P	0	0	0	36	32	0	0
03P	0	0	0	38	43	0	0
04P	0	0	0	43	39	0	0
05P	0	0	0	20	23	0	0
06P	0	0	0	33	27	0	0
07P	0	0	0	13	18	0	0
08P	0	0	0	14	13	0	0
09P	0	0	0	6	8	0	0
10P	0	0	0	5	5	0	0
11P	0	0	0	2	2	0	0
тот	0	0	0	381	423	53	0

# STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION BUREAU OF POLICY AND PLANNING PLANNING INVENTORY AND DATA

### TRAFFIC RECORDER DATA

TOWN OF MANSFIELD			ROUTE				DIRECTION B
CONANTVILLE RD #1 - NORTH OF DAY	POLLACK RE	) MON	TUE	WED	THU	FRI	SAT
DATE	0	10/03/2011	10/04/2011	0	0	0	0
TYPE	-						
HOUR							
	2011	ADT = 550	ACF = NA				
	******	*****					
12A	0	0	3	0	0	0	0
01A	0	0	2	0	0	0	0
02A	0	0	0	0	0	0	0
03A	0	0	1	0	0	0	0
04A	0	0	2	0	0	0	0
05A	0	0	2	0	0	0	0
06A	0	0	15	0	0	0	0
07A	0	31	0	0	0	0	0
08A	0	52	0	0	0	0	0
09A	0	39	0	0	0	0	0
10A	0	30	0	0	0	0	0
11 <b>A</b>	0	34	0	0	0	0	0
12P	0	43	0	0	0	0	0
01P	0	44	0	0	0	0	0
02P	0	36	0	0	0	0	0
03P	0	63	0	0	0	0	0
04P	0	52	0	0	0	0	0
05P	0	59	0	0	0	0	0
06P	0	27	0	0	0	0	0
07P	0	29	0	0	0	0	0
08P	0	17	0	0	0	0	0
09P	0	7	0	0	0	0	0
10P	0	6	0	0	0	0	0
11P	0	5	0	0	U	U	0
тот	0	574	25	0	0	0	0

# F.A. HESKETH AUTOMATED COUNT DATA

Meadowbrook Road west of Pollock Road Mansfield, CT Job No. 15131

F. A. Hesketh & Associates, Inc.
6 Creamery Brook
East Granby, CT 06026
Phone: (860) 653-8000

Date Start: 25-Jun-15 Date End: 01-Jul-15 Site Code: 11140001

Start	22-Jun-15	23-Jun-15	24-Jun-15	25-Jun-15	26-Jun-15	Weekday	27-Jun-15	28-Jun-15
Time	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun
12:00 AM	*	*	*	*	2	2	2	2
01:00	•	*	*	*	0	0	4	1
02:00		•	*	*	0	0	1	2
03:00	*	*	*	*	1	1	3	1
04:00	*	*	*	*	0	0	1	2
05:00	*	*	*	*	3	3	3	4
06:00	*	*	*	*	7	7	1	1
07:00	*	*	*	*	17	17	7	2
08:00	*	*	*	*	22	22	12	9
09:00	*	*	*	*	29	29	24	12
10:00	*	*	*	27	28	28	28	19
11:00	*	*	*	35	35	35	33	19
12:00 PM	*	*	*	40	36	38	21	33
01:00	•	*	*	24	33	28	29	20
02:00	*	*	*	41	27	34	36	24
03:00	*		*	36	31	34	17	21
04:00	*	*	*	36	40	38	21	27
05:00	*	*	*	40	35	38	26	19
06:00	*	*	*	34	37	36	15	15
07:00	*	*	*	28	36	32	11	18
08:00	*	•	*	13	20	16	16	11
09:00	*	*	*	8	12	10	12	4
10:00	•	*	•	9	10	10	5	3
11:00	*	*	*	2	4	3	2	3
Total	0	0	0	373	465		330	272
Percentage	0.0%	0.0%	0.0%	80.9%	100.9%		71.6%	59.0%
AM Peak	*	*	*	11:00	11:00		11:00	10:00
Vol.	-	-	-	35	35	•	33	19
PM Peak	-		~	14:00	16:00	*	14:00	12:00
Vol.	-	~		41	40	w	36	33

# F. A. Hesketh & Associates, Inc. 6 Creamery Brook East Granby, CT 06026

Meadowbrook Road west of Pollock Road Mansfield, CT Job No. 15131

Phone: (860) 653-8000

Date Start: 25-Jun-15 Date End: 01-Jul-15 Site Code: 11140001

Start	29-Jun-15	30-Jun-15	01-Jul-15	02-Jul-15	03-Jul-15	Weekday	04 <b>-J</b> ul-15	05-Jul-15
Time	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun
12:00 AM	0	0	1	*	*	0	*	
01:00	3	3	1	*	*	2	•	
02:00	0	0	0	*	*	0	*	
03:00	1	2	1	*	*	1	*	
04:00	1	0	0	*	*	0	*	
05:00	3	1	4	*	*	3	*	
06:00	8	13	10	*	*	10	*	
07:00	· 13	22	19	*	*	18	*	
08:00	22	23	20	*	*	22	*	
09:00	15	21	20	*	*	19	*	
10:00	39	39	17	*	*	32	*	
11:00	19	27	30	*	•	25	*	
12:00 PM	39	32	43	*	*	38	*	
01:00	33	39	38	*	*	37	*	
02:00	23	28	34	*	*	28	*	
03:00	31	41	36	*	*	36	*	
04:00	36	45	41	*	•	41	*	
05:00	28 "	33	36	*	*	32	*	
06:00	20	31	*	*	*	26	*	
07:00	22	29	*	*	*	26	*	
08:00	24	22	*	*	*	23	*	
09:00	18	13	*	*	*	16	*	
10:00	9	7	*	*	*	8	*	
11:00	1	5	*		*	3	•	
Total	408	476	351	0	0		0	(
ercentage	91.5%	106.7%	78.7%	0.0%	0.0%		0.0%	0.0%
AM Peak	10:00	10:00	11:00	······································	*	*	*	
Vol.	39	39	30	~	-	*	*	
PM Peak	12:00	16:00	12;00	-		-	•	
Vol.	39	45	43			*		
Total		476	351	<i>,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				

# F.A. HESKETH TURNING MOVEMENT COUNTS

Meadowbrook Lane at Pollack Road Mansfield, CT NO. 15131 East Granby, CT 06026 Phone: (860) 653-8000 Fax: (860) 844-860

File Name: Pollack AM1 Site Code: 15131100 Start Date: 6/25/2015

Page No : 1

44.4 38.9

08:45 AM

0.375

16.7

0.750

Groups Printed- Unshifted

gana araban-matata arabana araban	·		~····		·			s Printec	l- Unshi				**************************************		•		
			ck Road		M		orook La	ane			k Road		M		prook La	ane	
	ļ	Fron	י North			Fron	n East	A establishing an announce amendation or	ļ	From	South		ļ	From	ı West	na are a consequence and and share and	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	1	1	2	4
07:15 AM	0	0	0	0	0	1	0	1	0	0	0	0	1	0	0	1	2
07:30 AM	1	0	0	1	0	4	0	4	0	0	1	1	1	1	0	2	8
07:45 AM	1	0	0	1	0	1	0	1	0	0	0	0	0	4	0	4	6
Total	2	0	0	2	0	8	0	8	0	0	1	1	2	6	1	9	20
08:00 AM	1	0	0	1	0	2	0	2	1	0	1	2	0	1	2	3	8
08:15 AM	0	0	0	0	0	0	1	1	0	0	0	0	2	1	1	4	5
08:30 AM	0	0	2	2	0	0	0	0	0	0	0	0	2	3	0	5	7
08:45 AM	0	0	0	0	0	1	0	1	0	0	1	1	4	2	0	6	8
Total	1	0	2	3	0	3	1	4	1	0	2	3	8	7	3	18	28
Grand Total	3	0	2	5	0	11	1	12	1	0	3	4	10	13	4	27	48
Apprch %	60.0	0.0	40.0		0.0	91,7	8.3		25.0	0.0	75,0		37.0	48.1	14.8		
Total %	6.3	0.0	4,2	10,4	0.0	22.9	2.1	25.0	2.1	0.0	6.3	8.3	20.8	27.1	8.3	56.3	
		D-11								D-U	. D						
			k Road North		IVI		rook La 1 East	ıne			k Road South		IVI		prook La Nest	ine	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:00	AM to	08:45		k 1 of 1						1		L				
Intersection	08:00																
Volume	1	0	2	3	0	3	1	4	1	0	2	3	8	7	3	18	28
Percent	33,3	0.0	66.7		0.0	75.0	25.0		33.3	0.0	66.7		44.4	38.9	16.7		
08:45	0	0	0	_	_	1				_			4	•	0		
Volume	U	U	U	0	0	1	0	1	0	0	1	1	4	2	U	6	8
Peak Factor																	0.875
High Int.	08:30 /	ΑM			08:00	ΑM			08:00	٩M			08:45	AM		and the same of th	
Volume	0	0	2	2	0	2	0	2	1	0	1	2	4	2	0	6	
Peak Factor				0.375				0.500				0.375				0.750	
Peak Hour Fro	m 07:00	AM to	08:45 A	AM - Pea	ık 1 of 1												
By Approach	07:45	AΜ			07:00 /	ΔM			07:15	ΔM			08:00	AM			
Volume	2	0	2	4	0	8	0	8	1	0	2	3	8	7	3	18	

100.

0

0.0

33,3

08:00 AM

1

4

0.500

0.0 66.7

0.0

07:30 AM

0

2

0.500

0.0 50.0

Percent 50.0

Volume

Peak Factor

High Int. 08:30 AM

0

Route 195 at Meadowbrook Lane Mansfield, CT

Job No. 15131

East Granby, CT 06026 Phone: (860) 653-8000 Fax: (860) 844-860

File Name : Rt195 AM Site Code : 15131200 Start Date : 6/30/2015

Page No : 1

Groups Printed- Unshifted

arrana (arranga )		Route 195 rom North			Route 195 rom Soutl		Meac F			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App Total	Int Total
Factor	1.0	1.0		1.0	1.0		1.0	1.0		
07:00 AM	2	48	50	51	2	53	1	1	2	105
07:15 AM	4	47	51	63	1	64	1	2	3	118
07:30 AM	2	55	57	85	2	87	0	5	5	149
07:45 AM	7	77	84	101	1	102	0	4	4	190
Total	15	227	242	300	6	306	2	12	14	562
08:00 AM	5	65	70	104	4	108	1	4	5	183
08:15 AM	6	72	78	105	2	107	2	3	5	190
08:30 AM	3	99	102	95	0	95	4	4	8	205
08:45 AM	6	72	78	100	3	103	4	7	11	192
Total	20	308	328	404	9	413	11	18	29	770
Grand Total	35	535	570	704	15	719	13	30	43	1332
Apprch %	6,1	93.9	1	97.9	2.1		30.2	69.8		
Total %	2.6	40.2	42.8	52.9	1.1	54.0	1,0	2.3	3.2	

	1	Route 195 From North		1	Route 195 rom South			dowbrook From Wes		
Start Time	Right		App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour From 07:00	AM to 08:45	AM - Peak 1	of 1	***************************************			***************************************			
Intersection	MA 00:80									
Volume	20	308	328	404	9	413	11	18	29	770
Percent	6.1	93.9		97.8	2.2		37.9	62.1		
08:30 Volume	3	99	102	95	0	95	4	4	8	205
Peak Factor										0.939
High Int.	08:30 AM			08:00 AM			08:45 AM			
Volume	3	99	102	104	4	108	4	7	11	
Peak Factor			0,804			0,956			0 659	
Peak Hour From 07:00	AM to 08:45	AM - Peak 1	of 1							
By Approach	07:45 AM			08:00 AM			08:00 AM			
Volume	21	313	334	404	9	413	11	18	29	
Percent	6.3	93.7		97.8	2.2		37.9	62.1		
High Int.	08:30 AM			08:00 AM			08:45 AM			
Volume	3	99	102	104	4	108	4	7	11	
Peak Factor			0.819			0.956			0.659	

Meadowbrook Lane at Pollock Road

High Int. 04:45 PM

1 1

Volume

Peak Factor

Mansfield, CT Job No. 15131 East Granby, CT 06026

Phone: (860) 653-8000 Fax: (860) 844-860

File Name: Pollock PM Site Code: 2222222 Start Date: 7/1/2015

Page No : 1

													•	5-			
							Group:	s Printed	l- Unshi	fted							
		Polloc	k Road	errer errer errer errer er errer er	M	eadowb	rook La	ane		Polloc	k Road		M	eadowb	rook La	ne	
	1	From	North			Fron	n East			From	South			From	West		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Factor	1.0	1.0	1.0	ane are management	1.0	1,0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
04:00 PM	0	0	1	1	0	8	0	8	0	0	2	2	0	1	1	2	13
04:15 PM	1	0	0	1	1	3	0	4	0	1	1	2	0	4	1	.5	12
04:30 PM	0	0	1	1	0	2	0	2	0	0	1	1	2	4	0	6	10
04:45 PM	1	1	1	3	0	5	1	6	0	0	2	2	3	2	1	6	17
Total	2	1	3	6	1	18	1	20	0	,	6	7	5	11	3	19	52
05:00 PM	0	0	1	1	3	3	0	6	0	2	0	2	0	2	1	3	12
05:15 PM	1	0	1	2	2	4	0	6	0	0	1	1	2	2	0	4	13
05:30 PM	0	0	1	1	0	3	0	3	0	2	3	5	1	2	0	3	12
05:45 PM	0	0	1	1.	0	4	0	4	0	0	2	2	1	5	2	8	15
Total	1	0	4	5	5	14	0	19	0	4	6	10	4	11	3	18	52
rand Total	3	1	7	11	6	32	1	39	0	5	12	17	9	22	6	37	104
Apprch %	27.3	9.1	63.6		15.4	82.1	2.6		0.0	29.4	70.6		24.3	59.5	16.2		
Total %	2.9	1.0	6.7	10.6	5.8	30.8	1.0	37.5	0.0	4.8	11.5	16.3	8.7	21.2	5.8	35.6	

en e			ck Road n North		М		rook La 1 East	ine			k Road South		M		rook La West	ine	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	om 04:0	O PM to	05:45	PM - Pea	k 1 of 1				the state of the s							aria de la companya del la companya de la companya	
Intersection	04:45	PM															
Volume	2	1	4	7	5	15	1	21	0	4	6	10	6	8	2	16	54
Percent	28.6	14.3	57.1		23.8	71.4	4.8		0.0	40.0	60.0		37.5	50.0	12.5	1	
04:45	4	4	4	2	0	E	4	6	^	0	2	_	2	2	4		17
Volume		ı	1	3	0	5	1	6	0	0	2	2	3	2	1	6	17
Peak Factor																The state of the s	0.794
High Int.	04:45	PM			04:45	PM			05:30	PM			04:45	PM			
Volume	1	1	1	3	0	5	1	6	0	2	3	5	3	2	1	6	
Peak Factor				0.583				0.875				0.500				0.667	
Peak Hour Fro	m 04:00	0 PM to	05:45 I	PM - Pea	ık 1 of 1												
By Approach	04:30	PM			04:45	PM			04:45	PM			04:15	PM			
Volume	2	1	4	7	5	15	1	21	0	4	6	10	5	12	3	20	
Percent	28.6	14,3	57.1		23,8	71.4	4.8		0.0	40.0	60.0		25.0	60.0	15.0		

6

0.875

04:45 PM

0 5

1

3

0.583

05:30 PM

0

2

3

04:30 PM

2

0

6

0.833

5

0.500

Route 195 at Conantville Road Mansfield, CT East Granby, CT 06026 Phone: (860) 653-8000 Fax: (860) 844-860 File Name: Rt195 PM Site Code: 11111111 Start Date: 7/1/2015

Page No : 1

Job No. 15131

Groups Printed- Unshifted

Route 195

Route 195

				Groups Prin	ted- Unsh	ifted				
		Route 195 From North			Route 195 rom Sout			antville R rom Wes		
Start Time	Right	Thru	**************************************		OTTORIS CONTRACTOR STATE OF THE	a province and a contract of the contract of t	energy company and an arrangular area		App. Total	Int. Total
A contract of the contract of			App. Total	Thru	Left	App. Total	Right	Left	App. Total	IIII. TOIAI
Factor	1.0	1.0		1.0	1.0		1.0	1.0		
04:00 PM	8	113	121	101	3	104	5	3	8	233
04:15 PM	5	150	155	100	3	103	6	6	12	270
04:30 PM	1	139	140	125	1	126	3	8	11	277
04:45 PM	8	172	180	113	5	118	4	3	7	305
Total	22	574	596	439	12	451	18	20	38	1085
05:00 PM	9	132	141	107	0	107	1	9	10	258
05:15 PM	4	123	127	108	2	110	7	2	9	246
05:30 PM	3	122	125	107	0	107	6	1	7	239
05:45 PM	3	97	100	87	5	92	5	4	9	201
Total	19	474	493	409	7	416	19	16	35	944
Grand Total	41	1048	1089	848	19	867	37	36	73	2029
Apprch %	3.8	96.2	1	97.8	2.2		50.7	49.3	000.00	
Total %	2.0	51.7	53.7	41.8	0.9	42.7	1.8	1.8	3,6	
	····	Route 195			Route 195			antville R		
		From North			rom South			rom Wes	3	
Start Time	Right	Thru	App. Total	Thru	Left	App, Total	Right	Left	App. Total	Int Total
Peak Hour From 04:00 f	PM to 05:45	PM - Peak	1 of 1							
Intersection (	04:15 PM									
\ / = 1 =	00	500	040	4.45		45.4	4.4	00	40	1110

	1	From North			rom South			rom West		
Start Time	Riaht	Thru	App. Total	Thru	Left	App Total	Right	Left	App Total	Int. Total
eak Hour From 04:00	PM to 05.45			kaaraa aa a	ranar e e e e e e e e e e e e e e e e e e e		i		Americanismental de de man.	
Intersection										
Volume	23	593	616	445	9	454	14	26	40	1110
Percent	3.7	96.3		98.0	2.0		35.0	65.0		
04:45 Volume	8	172	180	113	5	118	4	3	7	305
Peak Factor										0.910
High Int.	04:45 PM			04:30 PM			04:15 PM			
Volume	8	172	180	125	1	126	6	6	12	
Peak Factor			0,856			0 901			0.833	
eak Hour From 04:00	PM to 05:45	PM - Peak	(1 of 1							
By Approach	04:15 PM			04:30 PM			04:15 PM			
Volume	23	593	616	453	8	461	14	26	40	
Percent	3.7	96.3		98.3	1.7		35.0	65.0		
High Int.	04:45 PM			04:30 PM			04:15 PM			
Volume	8	172	180	125	1	126	6	6	12	
Peak Factor			0.856			0.915			0.833	

# ITE TRIP GENERATION WORKSHEETS

# Average Rate Trip Calculations For 50 Dwelling Units of Apartments(220) - [E]

Project: Meadowbrook Gardens

Phase:

Open Date: Analysis Date:

Description:

	Average	Standard	Adjustment	Driveway
	Rate	Deviation	Factor	Volume
Avg. Weekday 2-Way Volume	8.53	0.00	1.00	427
7-9 AM Peak Hour Enter	0.11	0.00	1.00	6
7-9 AM Peak Hour Exit	0.45	0.00	1.00	22
7-9 AM Peak Hour Total	0.56	0.00	1.00	28
4-6 PM Peak Hour Enter	0.59	0.00	1.00	29
4-6 PM Peak Hour Exit	0.32	0.00	1.00	16
4-6 PM Peak Hour Total	0.90	0.00	1.00	45
Saturday 2-Way Volume	2.73	0.00	1.00	136
Saturday Peak Hour Enter	0.00	0.00	1.00	0
Saturday Peak Hour Exit	0.00	0.00	1.00	0
Saturday Peak Hour Total	0.79	0.00	1.00	40

The above rates were calculated from these equations:

```
24-Hr. 2-Way Volume:
                            T = 6.06(X) + 123.56, R^2 = 0.87
7-9 AM Peak Hr. Total: T = .49(X) + 3.73
R^2 = 0.83, 0.2 Enter, 0.8 Exit 4-6 PM Peak Hr. Total: T = .55(X) + 17.65
                           R^2 = 0.77, 0.65 Enter, 0.35 Exit
T = .54(X) + 2.45
AM Gen Pk Hr. Total:
                           R^2 = 0.82, 0.29 Enter, 0.71 Exit

T = .6(X) + 14.91

R^2 = 0.8, 0.61 Enter, 0.39 Exit
                                            0.29 Enter, 0.71 Exit
PM Gen Pk Hr. Total:
                           T = 7.85(X) + -256.19, R^2 = 0.85
Sat. 2-Way Volume:
Sat. Pk Hr. Total:
                           T = .41(X) + 19.23
                           R^2 = 0.56, 0 Enter, 0 Exit
T = 6.42(X) + -101.12, R^2 = 0.82
Sun. 2-Way Volume:
Sun. Pk Hr. Total:
                           R^2 = 0, 0 Enter, 0 Exit
```

Note: A zero indicates no data available. Source: Institute of Transportation Engineers Trip Generation Manual, 9th Edition, 2012

TRIP GENERATION 2013, TRAFFICWARE, LLC

### Average Rate Trip Calculations For 90 Dwelling Units of Apartments(220) - [E]

Project: Meadowbrook Gardens

Phase:

Open Date: Analysis Date:

Description:

	Average	Standard	Adjustment	Driveway
	Rate	Deviation	Factor	Volume
Avg. Weekday 2-Way Volume	7.43	0.00	1.00	669
7-9 AM Peak Hour Enter	0.11	0.00	1.00	10
7-9 AM Peak Hour Exit	0.43	0.00	1.00	38
7-9 AM Peak Hour Total	0.53	0.00	1.00	48
4-6 PM Peak Hour Enter	0.48	0.00	1.00	44
4-6 PM Peak Hour Exit	0.26	0.00	1.00	23
4-6 PM Peak Hour Total	0.75	0.00	1.00	67
Saturday 2-Way Volume	5.00	0.00	1.00	450
Saturday Peak Hour Enter	0.00	0.00	1.00	0
Saturday Peak Hour Exit	0.00	0.00	1.00	0
Saturday Peak Hour Total	0.62	0.00	1.00	56

The above rates were calculated from these equations:

```
T = 6.06(X) + 123.56, R^2 = 0.87
24-Hr. 2-Way Volume:
7-9 AM Peak Hr. Total: T = .49(X) + 3.73
```

 $R^2 = 0.83$ , 0.2 Enter, 0.8 Exit

4-6 PM Peak Hr. Total: T = .55(X) + 17.65

 $R^2 = 0.77$ , 0.65 Enter, 0.35 Exit

T = .54(X) + 2.45AM Gen Pk Hr. Total:

 $R^2 = 0.82$ , 0.29 Enter, 0.71 Exit T = .6(X) + 14.91  $R^2 = 0.8$ , 0.61 Enter, 0.39 Exit T = 7.85(X) + -256.19,  $R^2 = 0.85$ PM Gen Pk Hr. Total:

Sat. 2-Way Volume:

Sat. Pk Hr. Total: T = .41(X) + 19.23

 $R^2 = 0.56$ , 0 Enter, 0 Exit Sun. 2-Way Volume:  $T = 6.42(X) + -101.12, R^2 = 0.82$ 

Sun. Pk Hr. Total:  $R^2 = 0$ , 0 Enter, 0 Exit

Note: A zero indicates no data available. Source: Institute of Transportation Engineers Trip Generation Manual, 9th Edition, 2012

TRIP GENERATION 2013, TRAFFICWARE, LLC

# CAPACITY ANALYSIS WORKSHEETS

# **BACKGROUND TRAFFIC VOLUMES**

	مر	>	4	Î	1	4			
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	k#			4	Þ		 		 
Volume (veh/h)	19	12	9	420	320	21			
Sign Control	Stop			Free	Free				
Grade	0%			0%	0%				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Hourly flow rate (vph)	21	13	10	457	348	23			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type				None	None				
Median storage veh)									
Upstream signal (ft) pX, platoon unblocked									
vC, conflicting volume	835	359	371						
vC1, stage 1 conf vol	000	333	3/ (						
vC2, stage 2 conf vol									
vCu, unblocked vol	835	359	371						
tC, single (s)	6.4	6.2	4.1						
tC, 2 stage (s)									
tF (s)	3.5	3.3	2.2						
p0 queue free %	94	98	99						
cM capacity (veh/h)	335	685	1188						
Direction, Lane#	EB 1	NB 1	SB 1						
Volume Total	34	466	371						
Volume Left	21	10	0						
Volume Right	13	0	23						
cSH	417	1188	1700						
Volume to Capacity	0.08	0.01	0.22						
Queue Length 95th (ft)	7	1	0						
Control Delay (s) Lane LOS	14.4	0.3	0.0						
Approach Delay (s)	B 14.4	A 0.3	0.0						
Approach LOS	14.4 B	0.3	0.0						
Intersection Summary	J								
Average Delay			0.7	**					
Intersection Capacity Utilization	า		39.3%	10	U Level of	Service		Α	
Analysis Period (min)	•		15	,0	O LOVE! U	OCI VICE		Λ	

	<i>&gt;</i>		*	<b>*</b>	4	4	4	Ŷ	p	<b>\</b>	<b>↓</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations Volume (veh/h) Sign Control	3	<b>♣</b> 7 Free	8	1	<b>4</b> 3 Free	0	2	<b>♣</b> 0 Stop	1	2	<b>↔</b> 0 Stop	1
Grade		0%			0%			0%	0.00	2.00	0%	0.00
Peak Hour Factor Hourly flow rate (vph) Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage	0.92 3	0.92 8	0.92 9	0.92	0.92 3	0.92 0	0.92	0.92	0.92	0.92	0.92 0	0.92
Right turn flare (veh) Median type Median storage veh) Upstream signal (ft)		None 760			None							
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol	3	700		16			25	24	12	25	28	3
vCu, unblocked vol tC, single (s)	3 4.1			16 4.1			25 7.1	24 6.5	12 6.2	25 7.1	28 6.5	3 6.2
tC, 2 stage (s) tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	100	100	100
cM capacity (veh/h)	1619			1601			983	867	1069	983	862	1081
Direction, Lane#	EB 1	WB1	NB 1	SB 1								
Volume Total	20	4	3	3				,,,,				
Volume Left	3	1	2	2								
Volume Right	9	0	1010	1								
cSH Volume to Capacity	1619 0.00	1601 0.00	1010 0.00	1013 0.00								
Queue Length 95th (ft)	0.00	0.00	0.00	0.00								
Control Delay (s)	1.2	1.8	8.6	8.6								
Lane LOS	Α	A	A	A								
Approach Delay (s) Approach LOS	1.2	1.8	8.6 A	8.6 A								
Intersection Summary												
Average Delay Intersection Capacity Utiliza Analysis Period (min)	ation		2.9 13.3% 15	IC	U Level o	f Service			Α			

	۶	*	4	<b>†</b>	1	4		
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W			ર્લ	î»			
Volume (veh/h)	27	15	9	463	617	24		
Sign Control	Stop			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	29	16	10	503	671	26		
Pedestrians					16			
Lane Width (ft)					12.0			
Walking Speed (ft/s)					4.0			
Percent Blockage					1			
Right turn flare (veh)								
Median type				None	None			
Median storage veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	1223	684	697					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	1223	684	697					
tC, single (s)	6.4	6.2	4.1					
tC, 2 stage (s)			•					
tF (s)	3.5	3.3	2.2					
p0 queue free %	85	96	99					
cM capacity (veh/h)	193	449	899				***************************************	
Direction, Lane #	EB 1	NB 1	SB 1					
Volume Total	46	513	697					
Volume Left	29	10	0					
Volume Right	16	0	26					
cSH Valuma ta Camasibi	243	899	1700					
Volume to Capacity	0.19 17	0.01	0.41 0					
Queue Length 95th (ft)	23.2	1 0.3	0.0					
Control Delay (s) Lane LOS	23.2 C	U.3 A	0.0					
Approach Delay (s)	23.2	0.3	0.0					
Approach LOS	23.2 C	u.s	0.0					
Intersection Summary								
Average Delay			1.0				 	
Intersection Capacity Utiliza	ation		43.9%	IC	U Level o	f Service	Α	
Analysis Period (min)			15					

	Þ		7	1	4	4	4	Î	<i>&gt;</i>	1	1	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	_	4	_		4	_	_	4	•		4	
Volume (veh/h)	2	_ 9	6	1	_ 16	5	6	4	0	4	1	2
Sign Control		Free			Free			Stop			Stop	
Grade	0.00	0%	0.00	0.00	0% 0.92	0.00	0.92	0% 0.92	0.92	0.92	0% 0.92	0.92
Peak Hour Factor Hourly flow rate (vph)	0.92 2	0.92 10	0.92 7	0.92 1	0.92 17	0.92 5	0.92 7	0.92 4	0.92	0.92 4	0.92	0.92
Pedestrians	2	10	,		17	3	,	4	U	7	,	2
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)		760										
pX, platoon unblocked												
vC, conflicting volume	23			16			42	42	13	42	43	20
vC1, stage 1 conf vol												
vC2, stage 2 conf vol				40			40	40	40	40	40	00
vCu, unblocked vol	23			16			42	42	13	42 7.1	43 6.5	20 6.2
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	0.0	0.2
tC, 2 stage (s) tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			99	99	100	100	100	100
cM capacity (veh/h)	1592			1601			956	848	1067	956	847	1058
Direction, Lane#	EB1	WB 1	NB 1	SB 1				• • •				
Volume Total	18	24	11	<u> </u>								
Volume Left	2	1	7	4								
Volume Right	7	5	0	2								
cSH	1592	1601	910	965								
Volume to Capacity	0.00	0.00	0.01	0.01								
Queue Length 95th (ft)	0	0	1	1								
Control Delay (s)	0.9	0.3	9.0	8.8								
Lane LOS	Α	Α	Α	Α								
Approach Delay (s)	0.9	0.3	9.0	8.8								
Approach LOS			Α	Α								
Intersection Summary												
Average Delay			3.1									
Intersection Capacity Utiliza	ation		13.3%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

# **COMBINED TRAFFIC VOLUMES**

	<b>^</b>	7	4	†	Ţ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y	***************************************		4	Þ		
Volume (veh/h)	34	16	10	420	320	25	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	37	17	11	457	348	27	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft) pX, platoon unblocked							
vC, conflicting volume	840	361	375				
vC1, stage 1 conf vol	040	301	313				
vC2, stage 2 conf vol							
vCu, unblocked vol	840	361	375				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	89	97	99				
cM capacity (veh/h)	332	683	1183				
Direction, Lane#	EB 1	NB 1	SB 1				
Volume Total	54	467	375				
Volume Left	37	11	0				
Volume Right	17	0	27				
cSH	398	1183	1700				
Volume to Capacity	0.14	0.01	0.22				
Queue Length 95th (ft)	12	1	0				
Control Delay (s) Lane LOS	15.5 C	0.3	0.0				
Approach Delay (s)	15.5	A 0.3	0.0				
Approach LOS	13.3 C	0.5	0.0				
• •	J						
Intersection Summary			1.1				
Average Delay Intersection Capacity Utilization	,		40.1%	10	:U Level o	f Convice	Α
Analysis Period (min)	1		40.1%	I.C.	O LEVEL O	I SELVICE	^

	٨	>	7	€	4	4	4	†	<i>&gt;</i>	/	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (veh/h)	3	26	14	1	_ 8	0	4	0	1	2	0	1
Sign Control		Free			Free			Stop			Stop	
Grade	0.00	0%	0.00	0.00	0%	0.00	0.00	0% 0.92	0.92	0.92	0% 0.92	0.92
Peak Hour Factor Hourly flow rate (vph)	0.92 3	0.92 28	0.92 15	0.92 1	0.92 9	0.92 0	0.92 4	0.92	0.92	0.92	0.92	0.92
Pedestrians	ა	20	10	1	9	U	4	U	1	2	U	ı
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked	0			40			54	53	36	54	61	9
vC, conflicting volume vC1, stage 1 conf vol	9			43			54	53	30	54	01	9
vC1, stage 1 conf vol												
vCu, unblocked vol	9			43			54	53	36	54	61	9
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	100	100	100
cM capacity (veh/h)	1611			1565			941	836	1037	941	828	1073
Direction, Lane#	EB 1	WB 1	NB 1	SB 1								
Volume Total	47	10	5	3								
Volume Left	3	1	4	2								
Volume Right cSH	15	0	1	1								
Volume to Capacity	1611 0.00	1565 0.00	958 0.01	981 0.00								
Queue Length 95th (ft)	0.00	0.00	0.01	0.00								
Control Delay (s)	0.5	0.8	8.8	8.7								
Lane LOS	A	A	A	A								
Approach Delay (s)	0.5	8.0	8.8	8.7								
Approach LOS			Α	Α								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utiliza	tion		13.3%	IC	U Level c	f Service			Α			
Analysis Period (min)			15									

	>	•	•	<b>←</b>	4	<i>&gt;</i>	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Ĥ			र्ग	γ		
Volume (veh/h)	22	7	7	8	25	25	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	24	8	8	9	27	27	
Pedestrians							
Lane Width (ft) Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			32		52	28	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol			00		50	00	
vCu, unblocked vol			32 4.1		52 6.4	28 6.2	
tC, single (s) tC, 2 stage (s)			4.1		0.4	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		97	97	
cM capacity (veh/h)			1581		952	1048	
Direction, Lane#	EB1	WB 1	NB 1				
Volume Total	32	16	54				
Volume Left	0	8	27				
Volume Right	8	0	27				
cSH	1700	1581	998				
Volume to Capacity	0.02	0.00	0.05				
Queue Length 95th (ft) Control Delay (s)	0 0.0	0 3.4	4 8.8				
Lane LOS	0.0	3. <del>4</del> A	6.6 A				
Approach Delay (s)	0.0	3.4	8.8				
Approach LOS	0.0	0.1	A				
Intersection Summary							
Average Delay			5.2				
Intersection Capacity Utiliza	tion		16.7%	ICI	U Level o	f Service	Α
Analysis Period (min)			15				

	٨	•	4	1	1	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	Description and the second second
Lane Configurations	W			ન	ĵ <sub>a</sub>		
Volume (veh/h)	36	17	13	463	617	41	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	39	18	14	503	671	45	
Pedestrians					16		
Lane Width (ft)					12.0		
Walking Speed (ft/s)					4.0		
Percent Blockage					1		
Right turn flare (veh)				None	Mana		
Median type Median storage veh)				None	None		
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	1240	693	715				
vC1, stage 1 conf vol	12.10	000					
vC2, stage 2 conf vol							
vCu, unblocked vol	1240	693	715				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	79	96	98				
cM capacity (veh/h)	188	443	885				
Direction, Lane#	EB1	NB 1	SB 1				
Volume Total	58	517	715				
Volume Left	39	14	0				
Volume Right	18	0	45				
cSH	230	885	1700				
Volume to Capacity	0.25	0.02	0.42				
Queue Length 95th (ft)	24 25.8	1 0.5	0 0.0				
Control Delay (s) Lane LOS	20.6 D	0.5 A	0.0				
Approach Delay (s)	25.8	0.5	0.0				
Approach LOS	20.0 D	0.0	0.0				
	J						
Intersection Summary			1.3				
Average Delay Intersection Capacity Utilizati	on		45.0%	10	CU Level o	of Sarvice	Α
Analysis Period (min)	OH		45.0%	ic	C LEAGI C	N OCIVICE	^
Analysis Fellou (IIIII)			13				

(MANUS MANUS M	<i>)</i>		7	1	4-	•	4	Ť	P	1	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		♣			4			4	_		4	_
Volume (veh/h)	2	20	10	1	37	5	14	4	0	4	1	2
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%		0.00	0%	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	22	11	1	40	5	15	4	0	4	1	2
Pedestrians												
Lane Width (ft) Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)		110110			710110							
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	46			33			79	79	27	79	82	43
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	46			33			79	79	27	79	82	43
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)	0.0			0.0			3.5	4.0	3.3	3.5	4.0	3.3
tF (s) p0 queue free %	2.2 100			2.2 100			ა.၁ 98	4.0 99	3.3 100	100	100	100
cM capacity (veh/h)	1562			1579			905	809	1048	905	806	1027
							900	003	1040	300	000	1021
Direction, Lane # Volume Total	EB 1 35	WB 1 47	NB 1	SB 1 8								
Volume Lotal Volume Left	35 2		20 15	4								
Volume Right	11	1 5	0	2								
cSH	1562	1579	882	920								
Volume to Capacity	0.00	0.00	0.02	0.01								
Queue Length 95th (ft)	0.00	0.00	2	1								
Control Delay (s)	0.5	0.2	9.2	8.9								
Lane LOS	Α	Α	Α	Α								
Approach Delay (s)	0.5	0.2	9.2	8.9								
Approach LOS			Α	Α								
Intersection Summary												
Average Delay			2.5			-						
Intersection Capacity Utilizati	on		13.3%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

	-	*	1	<b>4</b>	4	<b>/</b>	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			લી	Ŋ		
Volume (veh/h)	19	29	29	28	15	15	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	21	32	32	30	16	16	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s) Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)				7,01,10			
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			52		130	36	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol						••	
vCu, unblocked vol			52		130	36	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s) tF (s)			2.2		3.5	3.3	
p0 queue free %			98		98	98	
cM capacity (veh/h)			1554		847	1036	
Direction, Lane#	EB 1	WB1	NB 1				
Volume Total	52	62	33	***************************************			
Volume Left	0	32	16				
Volume Right	32	0	16				
cSH	1700	1554	932				
Volume to Capacity	0.03	0.02	0.03				
Queue Length 95th (ft)	0	2	3				
Control Delay (s)	0.0	3.8	9.0				
Lane LOS		A	A				
Approach Delay (s)	0.0	3.8	9.0				
Approach LOS			Α				
Intersection Summary			<i></i>				
Average Delay	4.		3.6	10		f 0	<b>A</b>
Intersection Capacity Utiliza	ition		19.7%	IC	U Level o	r Service	Α
Analysis Period (min)			15				